

RISA Webinar

Understanding and Optimizing Hot Rolled Steel Design in RISA

Presenter: Matt Brown, P.E.





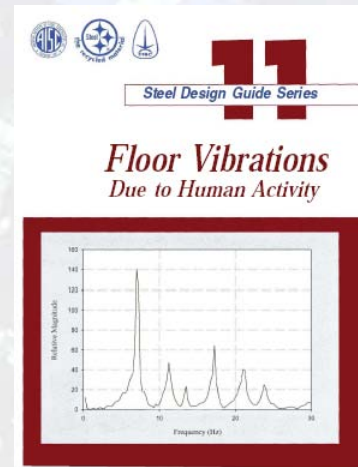
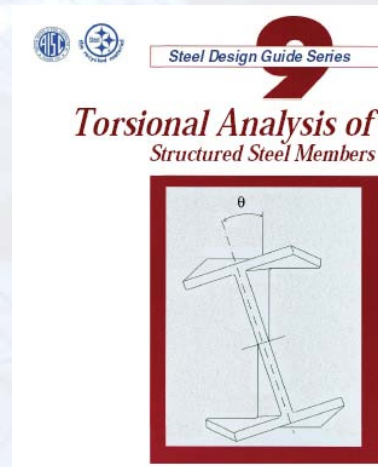
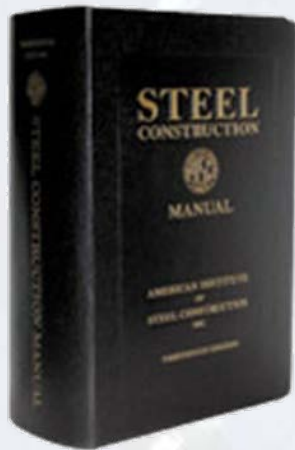
RISAFloor 5.1



RISA-3D 9.1

INTEGRATED PROGRAMS

- AISC 360-05 (13th Edition Steel Construction Manual)
- AISC Design Guide #9 (Torsion)
- AISC Design Guide #11 (Vibration)



Images courtesy of AISC.org

REFERENCED CODES

Today's Topics

- Direct Analysis Method
- Warping Torsion
- Single Angles
- Floor Vibrations
- Steel Joists

AISC Specification Chapter C
Stability Analysis and Design



AISC Specification Appendix 7
Direct Analysis Method

DIRECT ANALYSIS METHOD

Direct Analysis Requirements

1. Second Order Analysis ($P-\Delta$, $P-\delta$)
2. Stiffness Reduction (EI^* , EA^*)
3. Notional Loads

Cantilever Column

- W12x45
- 200k Gravity Load
- 10k Lateral Load
- 12'-0" Tall



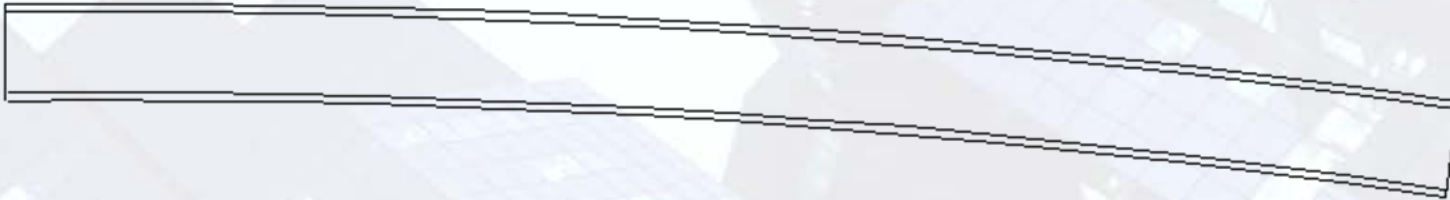
EXAMPLE PROBLEM

Initial Shear: 10k

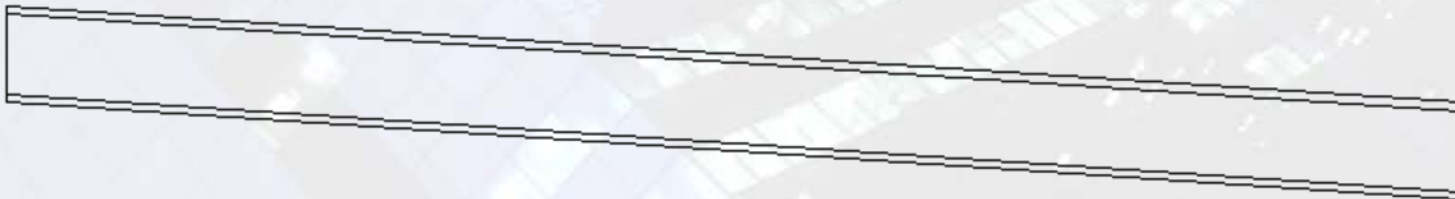
Initial Moment: $(10k) * (12 \text{ ft}) = 120 \text{ ft-k}$

Initial Deflection: $\frac{PL^3}{3EI} = 0.981''$

Bending Deflection: $\frac{PL^3}{3EI} = 0.981''$



Shear Deflection: $\frac{cPL}{A_wG} = 0.035''$



Final Shear: 11.6k
Final Moment: 139 ft-k
Final Deflection: 1.182"

16% Increase over First-Order

Final Shear: 12.5 k
Final Moment: 140 ft-k
Final Deflection: 1.216"

Direct Analysis Requirements

- ~~1. Second Order Analysis ($P-\Delta$, $P-\delta$)~~
2. Stiffness Reduction (EI^* , EA^*)
3. Notional Loads

Flexural Stiffness

$$EI^* = 0.8\tau_b EI$$

$$\tau_b = f \left(\frac{P_u}{P_n} \right)$$

Axial Stiffness

$$EA^* = 0.8EA$$

STIFFNESS REDUCTION

Direct Analysis Requirements

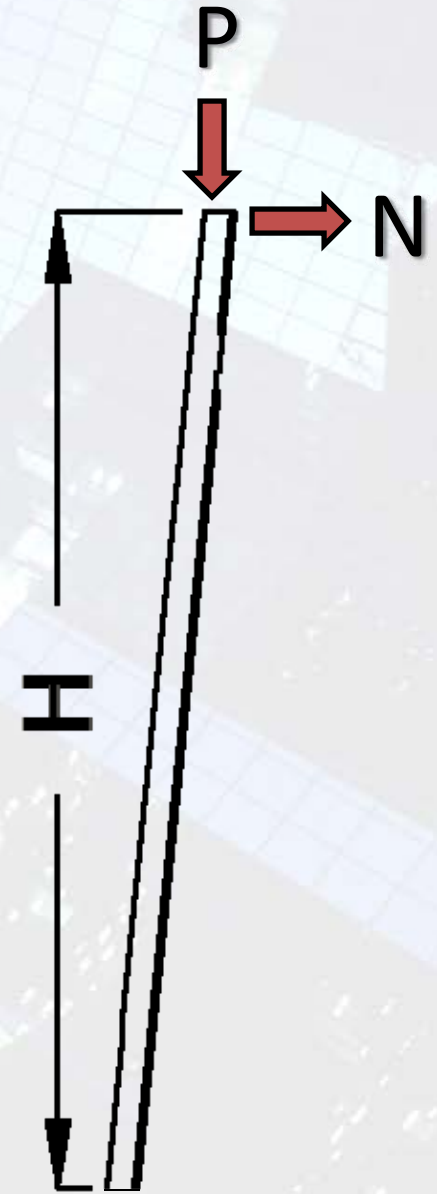
- ~~1. Second Order Analysis ($P-\Delta$, $P-\delta$)~~
- ~~2. Stiffness Reduction (EI^* , EA^*)~~
3. Notional Loads

Per AISC Code of Standard Practice

Erection Tolerance for Columns

(out of plumb) = $H/500$

Notional Load (N) = $P/500$



NOTIONAL LOADS

Direct Analysis Requirements

- ~~1. Second Order Analysis ($P-\Delta$, $P-\delta$)~~
- ~~2. Stiffness Reduction (EI^* , EA^*)~~
- ~~3. Notional Loads~~

$$K = 1.0$$

Traditional Design

$M = 120 \text{ k-ft}$

$K = 2.1$

Req'd Shape:

W12x40

Direct Analysis

$M = 152 \text{ k-ft}$

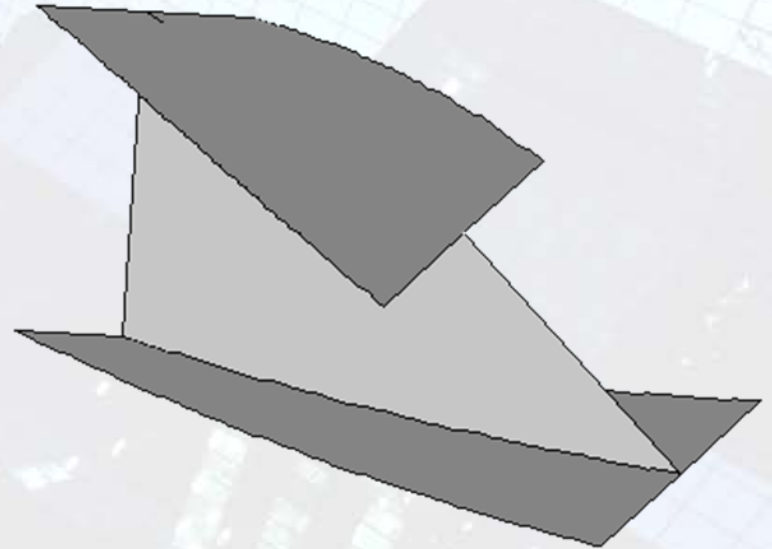
$K = 1.0$

Req'd Shape:

W12x45

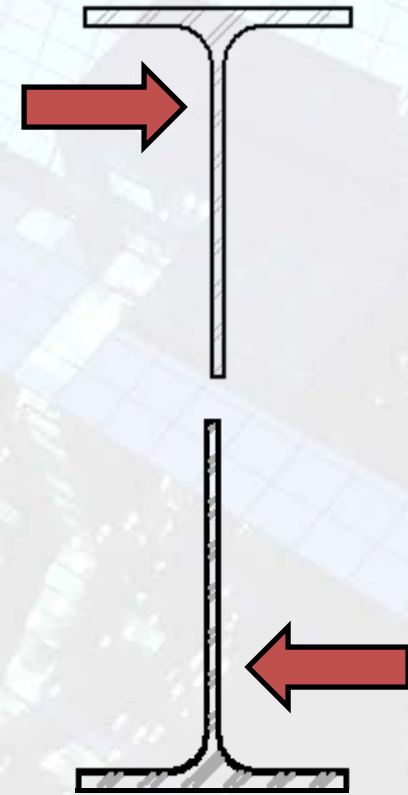
COMPARISON

Warping Torsion
affects Wide Flange
and Channel Shapes

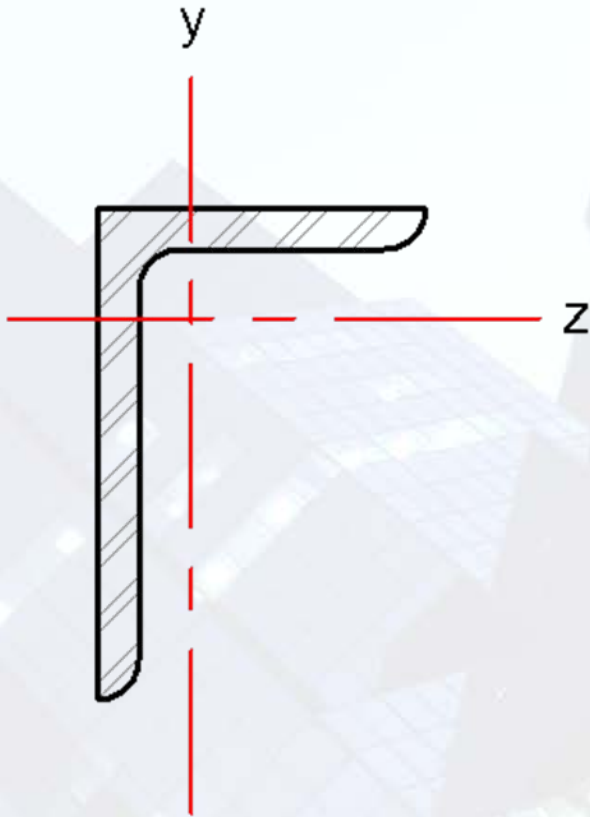


WARPING TORSION

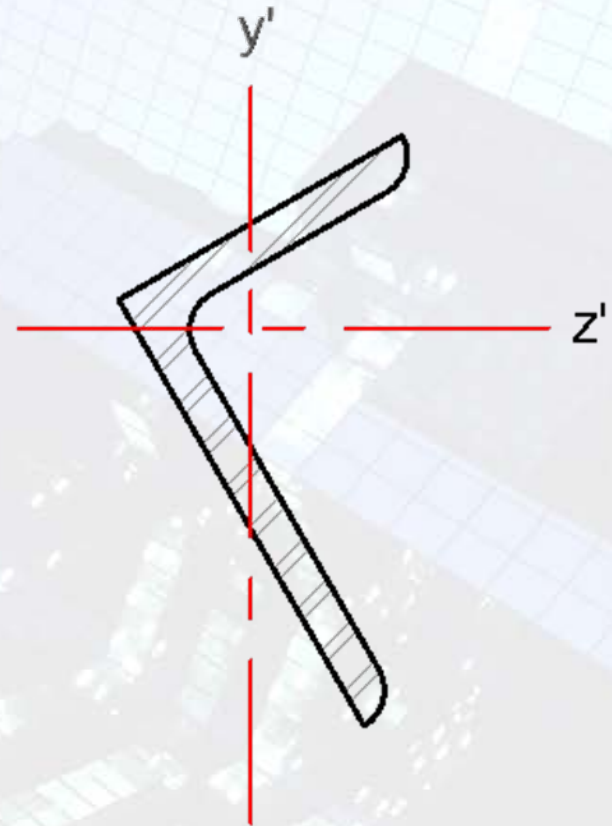
Idealize as two WT sections,
each in minor axis bending



WARPING TORSION



Geometric Bending



Principal Bending

SINGLE ANGLES

Floor Vibration Procedure:

1. Determine damping weight
2. Determine beam natural frequency
3. Calculate expected acceleration

Damping Weight

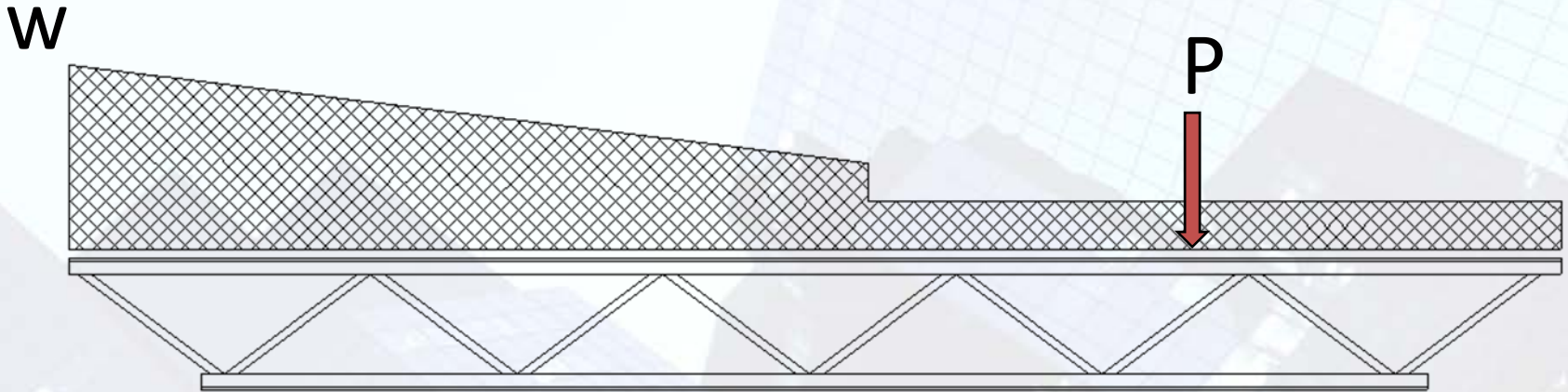
Beam Self Weight

+ Slab Weight

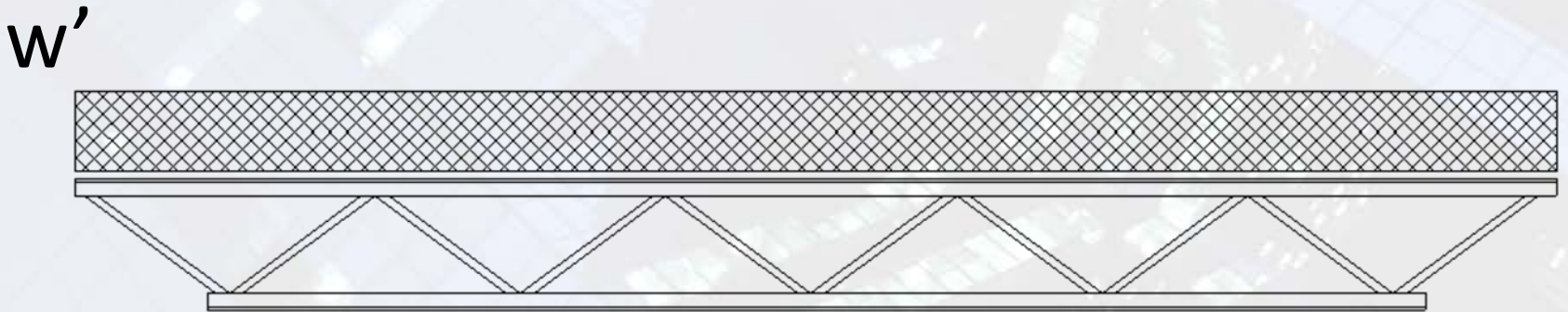
+ Realistic Superimposed Load (11 psf?)

Allowable Acceleration

- Based on 65 lb excitation force (footfall)
- 0.5% g for Offices, Residences, Churches
- 1.5% g for Shopping Malls



Actual Load



Equivalent Uniform Load

STEEL JOISTS

Additional Resources

- AISC Publications
- RISA-3D Help File / Manual
- www.risanews.com

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Questions?

Please let us know if you have questions.

We will answer as many questions as time permits during the webinar.

Once the webinar is closed, we will post all Q&A's, as well as the Quick Reference Guide, to our website: www.risa.com

For further information, contact us at: info@risatech.com

Thank you for Attending!

